ENGINEERING INVESTIGATION & ANALYSIS GEOTECHNICAL & STRUCTURAL ASSESSMENT REPORT

104 BIG PIECE ROAD FAIRFIELD, NEW JERSEY 08203

MATRIXNEWORLD

Engineering Progress

Prepared for:

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1.0 PROJECT BACKGROUND

The State of New Jersey Department of Community Affairs (DCA), Division of Disaster Recovery and Mitigation, anticipates receiving approval for grant funding through FEMA's Flood Mitigation Assistance (FMA) appropriation. This funding is provided through FMA to states and local communities to reduce or eliminate flood risk due to repetitive flood damage to buildings insured by the National Flood Insurance Program (NFIP). The DCA intends to use the funding for the State's Mitigation Assistance Program (MAP) to elevate residential properties located in a floodplain in the Township of Fairfield. The properties are to be elevated at least 3 feet above the base flood elevation (BFE). The DCA hosted a town hall meeting for homeowners in Fairfield, focused on homeowners with properties that experience Repetitive Losses or Severe Repetitive Losses.

In preparation of procuring a Design-Build firm to conduct the effort, the DCA has contracted Matrix New World Engineering, Land Surveying and Landscape Architecture, P.C. (Matrix) to conduct a geotechnical analysis, preliminary structural analysis, and elevation certificate for residences anticipated to be included in the program. It is understood that this document will serve as the basis for the development of a Request for Proposal (RFP) to procure Design-Build firms to do final structural design and perform the elevation of the properties.



2.0 PROJECT SCOPE

Matrix has completed a geotechnical and structural assessment and elevation certificate to evaluate the viability of elevating the residential building located at 104 Big Piece Road in Fairfield, New Jersey (Site). Matrix provided geotechnical and structural engineering and land surveying services as a consultant to the DCA. The project location is shown on the attached Site Location Map (Figure 1).

The purpose of the engineering study was to compile comprehensive data regarding the existing building's foundations and overall structural composition and condition at the Site. The information obtained will be further utilized to determine the feasibility and proposed design of raising the existing residence 3 feet above the base flood elevation (BFE) as determined by FEMA. A team of Matrix engineers and surveyors performed the evaluation, consisting of a geotechnical soil inspection, test pits to reveal the existing building foundations, an interior inspection of the building's visible foundation walls and frame, and topographic surveying for the development of a flood elevation certificate. A total of 2 test pits, (TP-1 and TP-2) were completed to depths of 56 and 49 inches, respectively, below the ground surface (bgs) and 2 geotechnical borings (B-1 and B-2) were completed to a depth of 27 feet bgs (see Figure 2).

Matrix's geotechnical characterization of the property is based on an engineering evaluation of the subsurface conditions as indicated by the field exploration data and geotechnical laboratory test results on representative soil samples.



3.0 SITE LOCATION & PROJECT DESCRIPTION

The project site is located at 104 Big Piece Road in Fairfield, New Jersey. The property consists of a 1.5-story timber-framed split-level house with an approximately 950 square foot footprint. The building is situated atop concrete masonry unit (CMU) foundation walls on cast-in-place concrete foundations. The substructure of the house is comprised of three different underground or ground-level areas, including a crawl space, a finished central basement area, and a ground-level area. The timber frame of the structure is covered with a vinyl siding throughout its exterior. The property also contains a timber-framed painted timber deck in the rear of the house which includes an enclosed sunroom with full length glass windows.

To assist with the geotechnical and structural evaluation, test pits and geotechnical borings were advanced in areas around the residence to obtain information regarding the soil's structural properties and building's existing foundation. The 2 test pits and 2 borings were located to provide the most useful information about the subsurface conditions. Refer to Figure 2 of this report for a map of the test pit and boring locations.



4.0 GEOLOGIC SETTING

According to the USDA Soil Survey of Essex County, the site is situated atop Pompton – Urban land. The subsurface composition is typically sand and loamy sands from 8 to 60 inches bgs.

According to the 2014 Bedrock Geologic Map of New Jersey, the Site is underlain by the Sedimentary and Bedded Volcanic Rocks Towaco Formation. Specifically, the subsurface consists of micaceous, reddish-brown sandstone, siltstone, and silty mudstone in upward-fining sequences. The Bedrock Geologic Map is shown in Figure 3.

From the Surficial Geologic Map of Northern New Jersey, compiled by and edited by Byron D. Stone, Scott D. Stanford, and Ron W. White in 2002, the natural surface material (beyond fill) is suggested to be in the Pine Brook terrace deposit, which contains sand and gravel, moderately to poorly sorted. The Surficial Geology map is shown in Figure 4.

The documented site conditions presented above are consistent with the findings from the subsurface investigation, in which loamy Sands were encountered throughout the length of the boring. Groundwater is expected between 4 and 6 feet bgs based on soil saturation levels. Bedrock was not encountered during this subsurface program.



5.0 SUBSURFACE FIELD PROGRAM

The subsurface investigation was completed by generally accepted practices in the Geotechnical Engineering field and consisted of the advancement of 2 test pits and 2 Standard Penetration Test (SPT) borings using mud rotary drilling methods. Matrix retained Boring Brothers, Inc., located in Egg Harbor Township, NJ, to complete the subsurface field program.

A Matrix Geotechnical Engineer provided full-time drilling oversight, soil logging, and sample collection. Matrix prepared the field test pit and boring logs, which included sample depths, SPT-N blow counts, soil recovery, and soil descriptions based on the Burmister Soil Classification System followed by the Unified Soil Classification System (USCS) letter symbol. Test pit and soil boring logs are provided in Appendix A. Classification tables and charts used to determine the soil attributes are included in Appendix B.

Upon the completion of the field program, representative samples were subjected to geotechnical laboratory analyses. Laboratory results aided in soil classification and assessing the relevant engineering properties of the stratigraphic layers which were used in developing the revised geotechnical parameters outlined herein. Geotechnical laboratory reports are included in Appendix C.

5.1 Test Pits

On May 14, 2021, Boring Brothers completed a foundation survey which included 2 test pits, TP-1 (Northeast Corner) and TP-2 (East Wall) were completed to depths of 56 and 49 inches below the ground surface, respectively. The test pits were dug using a Bobcat E55 and shovel to prevent any damage to the existing building foundations. The exterior edge of the building footing was exposed at both locations to accurately measure the structure's dimensions, as well as to analyze the conditions of the concrete foundation.

The Matrix Geotechnical Engineer also observed the subsurface soil conditions encountered within the test pits, noting the type and composition of the soils surrounding and beneath the existing footings. All test pits were backfilled with the original soils upon completion of the test pit logs. No test pit samples were collected at the site for further analysis.



5.2 SPT Borings

On May 17, 2021, Boring Brothers advanced 2 geotechnical borings with a Mobile CME 55 track-mounted drill rig using mud rotary drilling techniques.

Split spoon (SS) samples were collected in accordance with *ASTM D-1586*, *Standard Method for Penetration Test and Split-Barrel Sampling of Soils*. A standard 2-inch outer diameter split spoon, two feet in length, was used to collect the soil samples. An automatic 140-pound hammer having a 30-inch drop was used to drive the split spoon sampler. As a part of boring observation, the SPT blow counts were recorded for the 0- to 6-inch interval, the 6- to 12-inch interval, the 12- to 18-inch interval and the 18- to 24-inch interval. The SPT N-values for design purposes are reported as the sum of the SPT N values observed for the above referenced 6- to 12-inch interval and the 12- to 18-inch interval that the split spoon sampler was driven.

The Matrix Geotechnical Engineer observed the split spoon samples and collected representative samples in sealed containers for further examination. All borings were continuously sampled to 12 feet bgs and at every subsequent 5-foot interval thereafter. The 2 borings were each advanced to a depth of 27 feet bgs. The borings were backfilled with soil cuttings and bentonite hole plug (if necessary) upon completion of the borehole.

5.3 Laboratory Testing

In addition to the field investigation, a laboratory testing program was conducted to determine additional pertinent engineering characteristics of representative samples of on-site soils. The laboratory testing program was performed in general accordance with applicable ASTM standard test methods and included physical/textural testing of representative samples of various strata.

Upon review of the boring logs, Matrix selected representative samples for laboratory testing. Laboratory testing of selected samples was completed by TerraSense, LLC, located in Totowa, New Jersey. The following table presents a summary of the testing program.

The results of the laboratory testing program were utilized to assist in developing geotechnical design parameters and recommendations, and are provided in Appendix C.



Table 5.3-1: Laboratory Testing Program

Test	Testing Procedure	Quantity Performed	Sample Locations and Depth Intervals
Water Content	ASTM D2216	5	B-1: 4-6', 15-17', 25-27' B-2: 4-6', 20-22'
Sieve Analysis	ASTM D422	1	B-1: 15-17'
Atterberg Limits	ASTM D4318	1	B-1: 25-27'
Percent Fines	ASTM D1140	2	B-1: 15-17' B-2: 4-6'
Combined Sieve & Hydrometer	ASTM D422	1	B-2: 20-22'



6.0 SUBSURFACE CONDITIONS

The subsurface conditions beneath the site can be characterized by the following stratigraphy, proceeding from the surface materials downward, unless noted otherwise below. Classification tables and charts used to determine the soil attributes are included in Appendix B.

Test Pits

Top of concrete was uncovered in TP-1 (Northeast Corner) at 48" bgs. The concrete protrudes 2" from the wall and extends 8" deep at this location.

In TP-2 (East Wall) the top of concrete was uncovered at 37" bgs. The concrete protrudes 6" from the wall and extends 12" deep at this location.

Surface Cover

The surface cover for boring B-1 and B-2 consisted of grass cover and topsoil, approximately 2-3 inches thick.

Stratum 1: Upper Silt (ML)

Beneath the surface cover in boring B-1, a soil layer was encountered consisting of brown-gray Silt with a large amount of fine Sand. This Silt layer extended from the bottom of the surface cover to 2 feet below the ground surface (bgs).

The SPT N-value in this layer was recorded at 7 blows per foot (bpf), which is indicative of loose Silt. The SPT N-values for Stratum 1 are summarized in the tables below.

Table 6.0-1: SPT N-Values for Stratum 1

Soil Boring Location	USCS Group Symbol	Depth Below	SPT
2011 201 mg 200 mism	eses ereap symbol	Ground Surface	N-Values
B-1	ML	0-2'	7

Stratum 2: Upper Sand (SM, SP)

Beneath the Silt layer (Stratum 1) in boring B-1, and beneath the surface cover in boring B-2, a granular soil layer was encountered consisting of coarse-to-fine Sand with varying amounts of Silt and coarse-to-fine Gravel. This granular layer extended to approximately 13.5 feet bgs in both borings.



The SPT N-values in this layer ranged from 7 to 19 bpf, which is indicative of loose to medium-dense Sand. The SPT N-values for Stratum 2 are summarized in the tables below.

Table 6.0-2: Loose SPT N-Values for Stratum 2

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values	
B-1	SM	2-4'	8	
B-2	SM	2-6'	7	

Table 6.0-3: Medium Dense SPT N-Values for Stratum 2

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	SM, SP	4-13.5'	13-17
B-2	SM	0-2'	19
D-2	SM, SP	6-13.5'	14-18

Stratum 3: Lower Silt (ML)

Beneath the granular soil of Stratum 2, a second layer of Silt was encountered with significant amounts of medium-to-fine Sand and traces or fine Gravel. This Silt layer extended from approximately 13.5 to 18.5 feet bgs in both borings.

The SPT N-values in this layer were recorded at 14 bpf in both borings, which is indicative of medium Silt. The SPT N-values for Stratum 3 are summarized in the tables below.

Table 6.0-4: SPT N-Values for Stratum 3

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	ML	13.5-18.5'	14
B-2	ML	13.5-18.5'	14

Stratum 4: Lower Sand (SM)

Beneath the Lower Silt (Stratum 3) in boring B-1, a second granular layer was encountered consisting fine Sand with some Silt and traces of fine Gravel. This layer extended from approximately 18.5 to 23.5 feet bgs.



The SPT N-value in this layer was recorded at 19 bpf, which is indicative of medium-dense Sand material. The SPT N-values for Stratum 4 are summarized in the tables below.

Table 6.0-5: SPT N-Values for Stratum 4

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	SM	18.5-23.5'	19

Stratum 5: Clay (CL)

Beneath the Lower Sand layer (Stratum 4) in boring B-1, and beneath Lower Silt layer (Stratum 3) in boring B-2, a soil layer was encountered consisting of brown to gray Clay with varying amounts of fine Sand. This cohesive layer was encountered at approximately 23.5 feet bgs in boring B-1 and at approximately 18.5 feet bgs in boring B-2. Both borings were terminated within this layer at 27 feet bgs.

The SPT N-values in this layer ranged 4 to 13 blows per foot (bpf), which is indicative of medium-soft to stiff Clay. The SPT N-values for Stratum 5 are summarized in the tables below.

Table 6.0-6: Medium-Soft SPT N-Values for Stratum 5

Soil Boring Location	USCS Group Symbol	Depth Below	SPT
Son Boring Eccucion	eses Group symbol	Ground Surface	N-Values
B-2	CL	18.5-23.5'	4

Table 6.0-7: Stiff SPT N-Values for Stratum 5

Soil Boring Location	USCS Group Symbol	Depth Below Ground Surface	SPT N-Values
B-1	CL	23.5-27'	10
B-2	CL	23.5-27'	13

Groundwater

Groundwater levels could not be measured during drilling in either boring, due to the use of water and drilling mud to advance the borings. Based on soil saturation levels, the groundwater table lies approximately between 4 to 6 feet bgs. It should be noted that the groundwater levels will vary with temperature, precipitation, and other climatic factors.



7.0 GEOTECHNICAL SUBSURFACE PARAMETERS

The geotechnical design parameters in this report are derived from the field program and are based on accepted geotechnical standards and practices. At the time of the geotechnical assessment, loading conditions and the final proposed grading plans were not available. Therefore, certain assumptions were made for the recommendations provided in this report.

Table 7.0-1 summarizes the recommended geotechnical design parameters for the various soil strata encountered at the Site. The values are based on review and interpretation of the subsurface field program and laboratory test data results.

Table 1806.2 of the 2018 International Building Code provides allowable coefficients of friction to be used in the evaluation of resistance to sliding.



Table 7.0-1: Geotechnical Design Parameters

	Unit	Friction Angle	Cohesive Strength,		Pressure Ticient	Net Allowable	Lateral	
Stratum	Weight	(Ф)	c_{u}	Active	Passive	Foundation Pressure*	Bearing	
	(pcf)	(deg)	(psf)	(Ka)	(Kp)	(psf)	(psf/ft. bgs)	
Native Medium-Dense to								
Dense Granular Soil	$\gamma = 125$	32°	0	0.31	3.26	4,000	200	
(SP, SP-SM, SM)	γ' = 63	32	U	0.51	3.20	4,000	200	
[SPT N > 10]								
Native Loose Granular Soil (SP, SP-SM, SM) [SPT N ≤ 10]	$\gamma = 120$ $\gamma' = 58$	30°	0	0.33	3.00	2,500	150	
Native Silt (ML) Loose [SPT N < 10]	$\gamma = 90$ $\gamma' = 28$	26°	150	0.39	2.56	1,500*	75	
Native Silt (ML) Medium [10 \(\leq\)SPT N \(\leq\)30]	$\gamma = 115$ $\gamma' = 53$	28°	400	0.36	2.77	2,000*	100	
Native Clay Material (CL) Medium Soft $[4 \le SPT \ N \le 8]$	$\gamma = 100$ $\gamma' = 38$	-	1,000	-	-	1,500*	75	
Native Clay Material (CL) Stiff [8 < SPT N \le 30]	$\gamma = 110$ $\gamma' = 48$	-	1,500	-	-	2,000*	100	

Notations: $\gamma = \text{moist unit weight}, \qquad \gamma' = \text{buoyant unit weight}, \text{ and } \qquad c_u = \text{average undrained shear strength}.$

- + Allowable foundation pressure is contingent upon either replacement of at least two feet of existing fill below the bottom of footing by a Controlled Fill, or upon confirmation that the field density of the existing fill material down to four feet below the bottom of footing meets 95% of the maximum dry density of the existing fill material observed in Modified Proctor Tests.
- * These values are based on the 2018 International Building Code, New Jersey Edition, and adjusted for field conditions encountered. To increase the allowable foundation pressure above the values recommended in the table given above, further testing of soil will be required. In Cohesive soils, it should be noted that the shallow footing may fail under the settlement criteria before the footing pressure approaches the anticipated allowable bearing capacity. Allowable Foundation Pressure values assume the water table is below the influence depth of the foundation.
- Coefficient of earth pressure at rest may be computed using Jaky's equation, $K_0 = 1 \sin \phi'$.



8.0 STRUCTURAL INSPECTION

The following sections present the results of the structural inspection of the residential building at 104 Big Piece Road in Fairfield, New Jersey. The conclusions presented herein are derived from Matrix's geotechnical and structural investigation of the existing soils and building foundations and framing configurations, along with pertinent survey data as compiled by Matrix's team of land surveyors.

Matrix conducted a subsurface investigation that included both test pits and soil borings to obtain maximum pertinent information regarding the existing site conditions (refer to Section 6.0 of this report). Each test pit performed at the site exposed the exterior portion of the building's foundation wall footings, allowing for measurement of dimensions of the structure and assessment of the construction methods utilized. Two geotechnical borings were also conducted to gain further information regarding the existing soils beneath the site.

In addition to the geotechnical investigation, Matrix also conducted a structural site inspection to observe the existing foundation walls and framing of the building. Matrix's structural engineer was granted access to the residence's basement and crawl space to observe the building's foundation structure. Substructure composition was recorded, including beam/girder type, building dimensions, and spacing of structural components. Structural defects, if any, were also noted during the inspection and have been included within Section 8.3.

8.1 Existing Building Foundations

The building at 104 Big Piece Road sits atop a crawl space area and two finished ground-level/basement areas. The timber frame and subfloor are supported by the CMU (8x8x18 block) foundation walls, as well as a nominal 2x8 timber girder in the crawl space area.

Encompassing the east portion of the house, beneath the first-floor level, is a rectangular crawl space with CMU foundation walls ranging from 53" to 54" in height. The first-floor surface above was measured at approximately 63.5" above the crawl space floor surface (surface is uneven). The subfloor of the level above is comprised of nominal 2x8 timber joists running north to south (front to rear of building). A support girder consisting of (3) nominal 2x8 timber beams runs perpendicular to the floor joists to provide additional support along the center of the crawl space. The girder bears on the CMU foundation walls at each end and is supported at its midspan by a CMU block column consisting of (6) stacked 8x8x18 blocks.



Two test pits were conducted around the perimeter of the east crawl space, in the northeast corner and along the east wall. Below the foundation walls in the crawl space area, an approximately 20" wide concrete spread footing was revealed during the test pit excavation program. Based on our findings within the test pits and from conventional foundation construction, Matrix utilized a 16" wide footing as a minimum value for analysis, but believes the actual footings for the building to likely range from 16" to 24" in width. Prior to raising the house, Matrix recommends that the contractor confirm the foundation size and bearing adequacy with multiple test pits around the building perimeter.

The remainder of the CMU foundation walls are covered in a plaster coating, as the basement and ground-level spaces contain three finished, habitable rooms in the split-level house. The foundation walls can be differentiated from the timber house frame by their protrusion approximately 6.5" into the building (thinner walls above). The floor of the central finished basement measured approximately 9" higher than the adjacent crawl space floor, and the west finished ground-level area was approximately 10.5" higher than the central basement. The floor of the west ground-level area is flush with the adjacent asphalt driveway in front of the property.

8.2 Existing Equipment

Within the laundry room located in the southern portion of the west ground-level area, a water heater was observed on a 4" high concrete block pad. A washer and dryer were also in this room, situated at the ground level. An electrical panel was noted within the closet of the north room of the west ground-level area, also at the ground level.

The only machinery observed within the crawl space was a sump pit located in the northwest corner of the area, near the entrance opening. Multiple metal and PVC pipes were also observed throughout the crawl space at varying elevations.

An air conditioning unit was observed outside the building along the east wall, situated atop a timber platform that raised the unit approximately 32" above the exterior grade. Also, a solar panel meter was observed on the rear wall of the building, attached to the timber frame.



8.3 Site Observations

The rear of the property at 104 Big Piece Road was subject to standing water at the time of the inspection. The eastern side of the backyard (between the two neighboring houses) was under water, as a stream running along the southern edge of the property appeared to have broken off its original pathway and created a marsh-like area within the property. No standing water was observed within the building at the time of the inspection.

Above the finished ground-level and basement rooms, the second floor was seen to extend approximately 24" further outward along the front of the building, creating a cantilevered overhang.

A cinder block chimney was observed protruding from the west wall of the crawl space, near the southwest corner. This chimney can be seen to extend up and through the roof of the first-floor level of the house.

A timber deck was observed spanning the full width of the building's rear wall. The deck's timber subfloor is supported by timber posts embedded in concrete Sonotube footings. The northeast corner of the deck, adjacent to the building, is enclosed with full length glass windows, creating a sunroom. The deck is approximately 24" below the adjacent first-floor surface on the east side, but is higher in the west half of the deck area.

Solar panels were observed covering the southwest corner of the building's roof at the time of the inspection.

8.4 Elevation Requirements

The FEMA 100-year flood elevation at 104 Big Piece Road is El. +174 (NAVD88). As per the New Jersey Department of Community Affairs (DCA), and in accordance with the New Jersey Flood Hazard Area Control Act, the lowest floor of newly elevated buildings must be at least 3 feet above the base flood elevation. Therefore, the new first floor elevation must be at El. +177 or higher to meet the requirements set forth in the program.

The current first-floor elevation at the Site is at El. +173.18, with the finished basement level at El. +168.15. To achieve the elevation requirements, the existing building would need to be raised at least 3.9 feet to elevate the existing first floor 3 feet above the BFE.



8.5 Recommendations for Building Elevation

Matrix recommends that the existing foundation system of the residential building at 104 Big Piece Road be kept and extended to achieve the required design flood elevation. The existing CMU foundation walls and concrete footings are expected to provide sufficient support for the additional height of the newly raised building. Based on loading estimation and analysis for the existing building, Matrix estimates that the anticipated additional dead load of the required new courses of CMU would remain under an allowable bearing capacity of 2,500 psf for the shallow concrete strip footings at the Site.

In accordance with NFIP requirements, it is required that the existing crawl space and central finished basement area be filled in to match the lowest adjacent exterior grade following raising. The newly raised house will have at least 7.25 feet of height throughout the ground-floor level, which can be used for storage at the resident's discretion. Raising the house in this manner will result in a loss of habitable area for the residence, as the existing ground-level and basement floors can no longer be used for living space (below the design flood elevation).

Alternatively, the homeowner may elect to raise the existing house an additional 4 feet (7.9 feet total above current elevation) and construct a new timber floor above the existing basement and ground level. The additional 4 feet of elevation will allow for the new first-floor ceiling height to be above the required limits for habitable space as per the 2018 International Residential Code, New Jersey Edition. Raising the house in this way will preserve the original square footage of the building's habitable space while also providing a new ground level for storage. A new load-bearing timber wall will be required above the existing CMU foundation wall on the new first floor to carry the load from the second floor down to the foundation. The existing foundation system of the building is expected to sufficiently support the additional loading from the raised walls and a new first floor, but footing size must be confirmed for these foundations prior to construction.

The most feasible method of elevation for the building consists of jacking up the entire residential structure from below using steel beams and jack posts. The building will then sit atop temporary cribbing while the existing CMU and concrete basement/crawl space walls are heightened with additional courses of masonry block units. Additional vertical reinforcement would need to be installed in ungrouted masonry cells to properly transfer loads through the new heightened wall to the existing wall, and horizontal ladder reinforcement should be installed at a minimum of every other course. Also, the existing concrete block pedestal supporting the building's crawl space girder must be removed and replaced by a new concrete or



masonry block column. The new column will need to include a spread footing beneath to sufficiently support the building loads. Additionally, the rear timber deck and covered porch are anticipated to require raising to match the current ingress/egress heights of the main structure. This would require replacement or extension of the timber support posts.

Within the new foundation walls, permanent openings are required to allow floodwater to enter the ground level and equalize the hydrostatic pressure during a flood event. As per the 2018 International Residential Code, New Jersey Edition, the total net area of non-engineered openings must comprise at least 1 square inch for every square foot of enclosed space within the building's ground floor. This equates to approximately 6.6 square feet of total flood openings in the building's new foundation walls. Additionally, a minimum of two openings must be provided for each enclosed area of the new ground floor. These openings must be located no higher than one foot above the adjacent finished exterior grade along the building perimeter. Matrix recommends the use of engineered openings in lieu of non-engineered openings to maximize efficiency and minimize the quantity of openings required.

Additionally, any service equipment, whether outside or in the crawl space/ground floor rooms, such as air conditioning, heat pump compressors, gas meters, electric meters, and hot water heaters, must be elevated 3 feet above the BFE. For interior elements, this may include relocation to an upper floor and thus less usable living space. For this residence, the hot water heater in the ground-floor laundry room and the electrical panel in the ground-floor west closet would require elevating 3 feet above the BFE onto the raised first floor. The exterior air conditioning unit would also require elevating 3 feet above the BFE on a new or extended exterior platform.



9.0 CLOSURE

This report has been prepared to assist the State of New Jersey Department of Community Affairs with the structural and geotechnical evaluation of the residential building 104 Big Piece Road in Fairfield, New Jersey. The conclusions and recommendations provided within this report were prepared based on our understanding of the project and through the application of generally accepted engineering practices. No warranties, expressed or implied, are made. Matrix should be notified of any changes to the existing building foundation system or if subsurface conditions differing from those described herein are encountered, so the impact on the geotechnical and/or structural recommendations can be evaluated.



10.0 REPRESENTATIVE SITE PHOTOS

Structural Inspection Photos



Photo 1. 104 Big Piece Road (Front of Building)



Photo 2. 104 Big Piece Road (Rear of Building)





Photo 3. Second Floor Cantilever Overhang in Front of House



Photo 4. Rear Sunroom on Timber Deck (Looking East)





Photo 5. East Crawl Space (Looking East)



Photo 6. Block Chimney East Crawl Space (West Wall)





Photo 7. Girder with CMU Pedestal in East Crawl Space (Looking West)



Photo 8. Sump Pit in East Crawl Space (Northwest Corner)



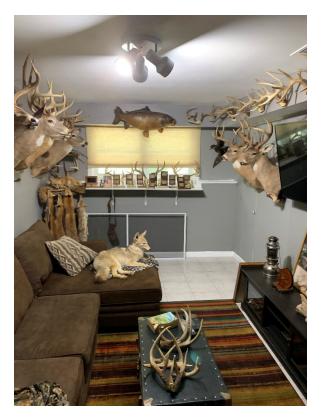


Photo 9. Central Finished Basement (Looking Northwest)



Photo 10. Water Heater in Ground-Level Laundry Room (Looking East)



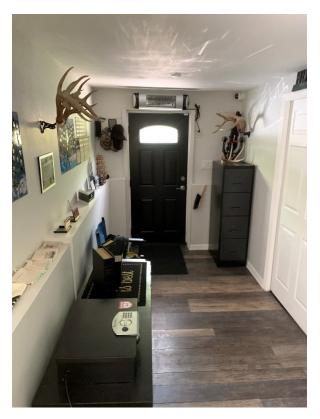


Photo 11. North Room of West Ground-Level Area (Looking Northwest)



Photo 12. Protrusion of CMU Foundations Walls into Building (Typical Around Perimeter)



Test Pit Photos



Photo 13. Test Pit TP-1 Location (Front of Building – Northeast Corner of Crawl Space)



Photo 14. Test Pit TP-1 Foundation Conditions



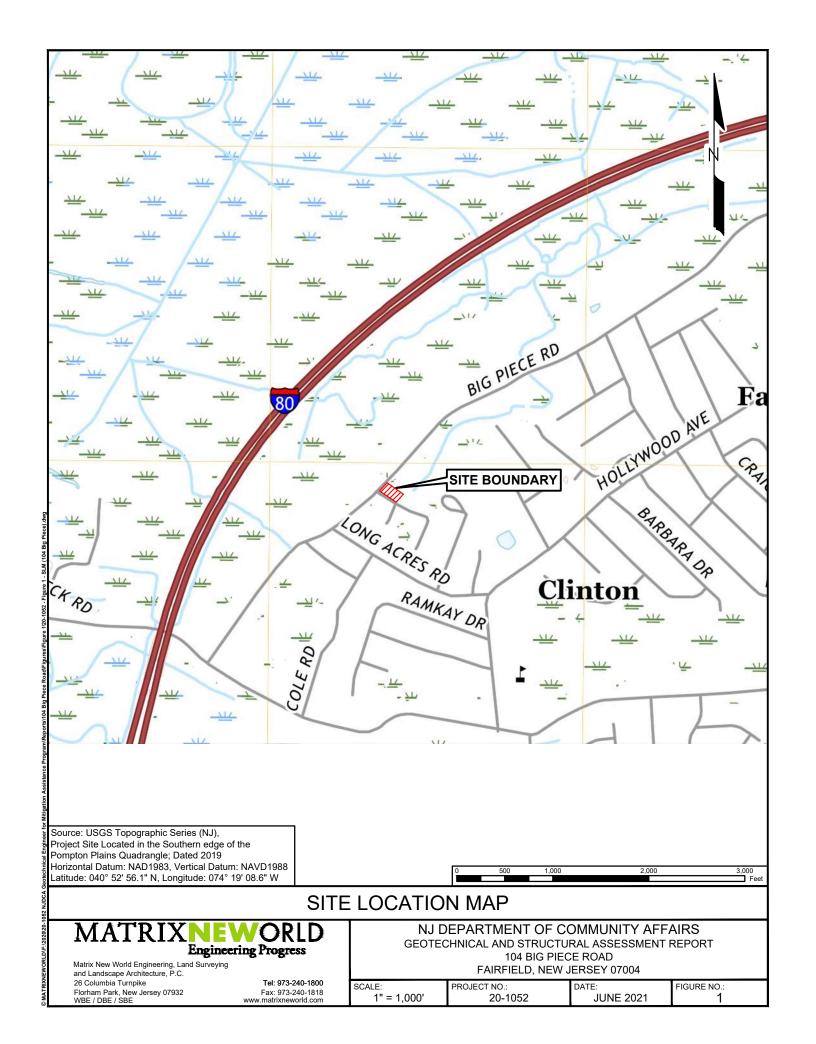


Photo 15. Test Pit TP-2 Location (East Wall of Building – Crawl Space)

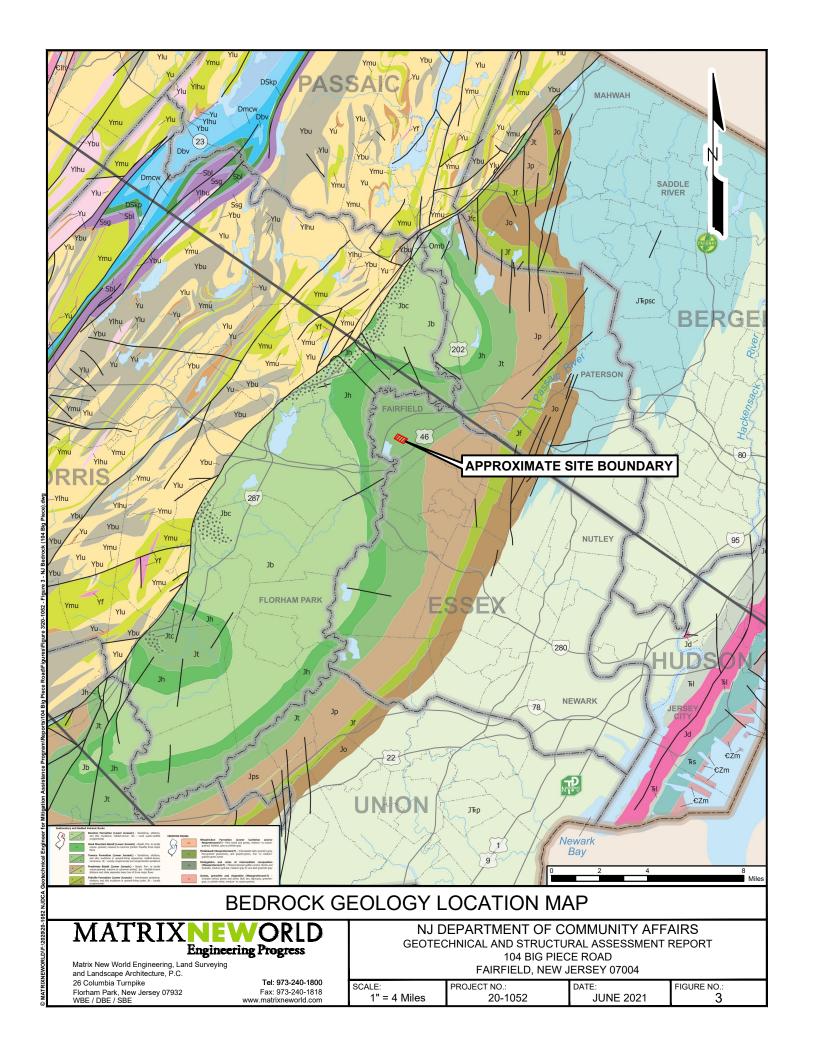


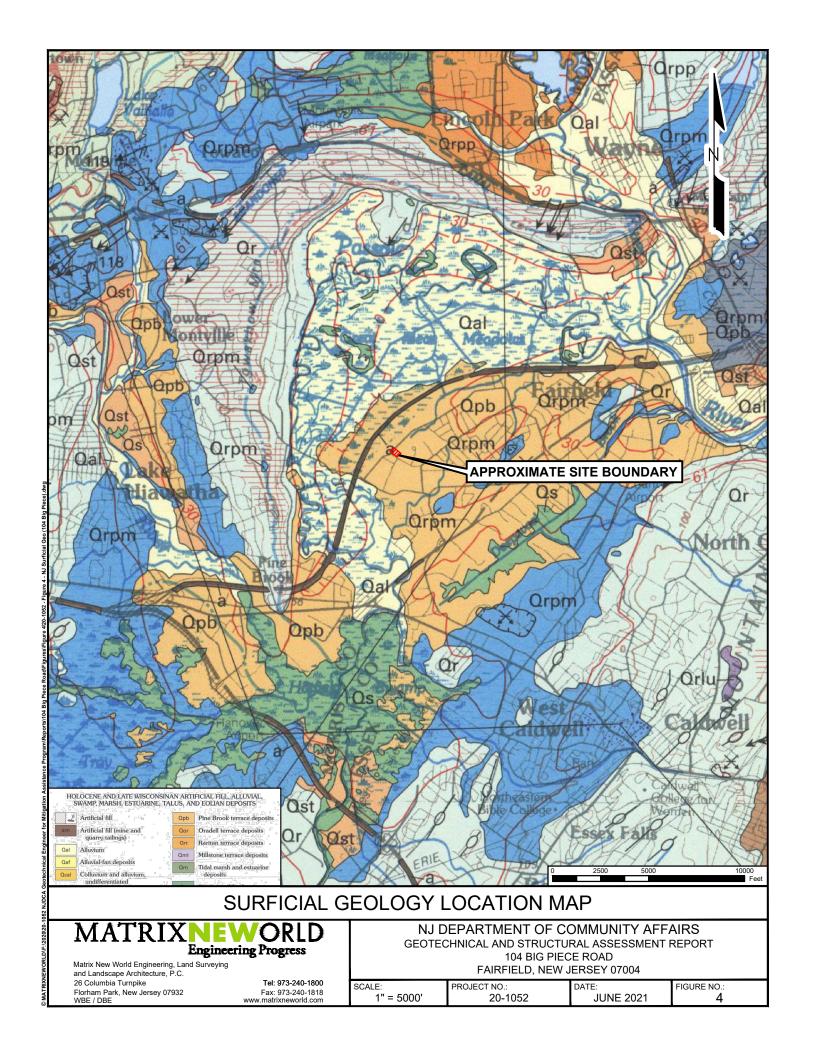
Photo 16. Test Pit TP-2 Foundation Conditions











APPENDIX A SOIL BORING & TEST PIT LOGS



Engineering Progress

BORING LOG

												BORIN	IG NO.:	B-1	
												SHEE	г <u>1</u>	OF1	
PROJEC	CT NO.: _	20-	1052	_ PRO	JECT: N	NJDO	CA Geo	otechnical	Engineer for	Mitigation A	Assistance	Program DA	TE:	5/17/21	
PROJEC	CT LOCAT	ION:			Fairfield	d, NJ	l	E	ORING LOCA	ATION:	104 Big Pi	ece Road, No	ortheast S	ide of Hous	se
DRILLIN	IG EQUIPI	MENT:	<u></u>	CME	55	AN	GLE:	90.0 D	DIR.:	ELE	V.:	DATU	M:	NAVD88	
	IG CONTR								RILLER:			INSPECTO	DR:	A. Radiola	
	CASI	NG and	HAMN	/ER				SAMDI ER	and HAMMER			GROUNDWA	ATER LEVE	18	
Туре			Weig		Drop	-	Гуре	I.D.	Weight	Drop	Date	Time	Depth	Casing De	 epth
Auto		-	140	_	30"		UTO		140 lbs	30"					F
FJ Ste	el 4'	•					SS	1 3/8"							
Depth	CASING			SAMPLE			l								
						/0"	- je je		D		Of Matan	ial		Laborato	ory
Feet (Elev.)	Blows/ Foot	No.	Туре	Depth Feet	Blows (REC.	%)	Graphic Symbol		De	escription	Oi Mater	ıaı		Tests	
(LIEV.)	1 001	0.4			[RQD		<u> </u>	0.4.5	0 00.7						
_		S-1	SS	0-2	4-4-3 (58%			S-1: Brov	vn-Grey SILT	and fine Sar	nd, mottling	, dry (ML)			
-					(007	٠,									
_		S-2	SS	2-4	2-4-4	l-4		S-2: Grey	/-Brown fine S	AND and Si	ilt, mottling,	trace roots, o	dry (SM)	_	
_					(67%	6)		_			•		, ,		
_														0.	
5		S-3	SS	4-6	10-8-9			S-3: Brov	vn mf* SAND, 5%, Gravel: 0.3	some Silt, v 3%_Sand: 7	vet (SM) 2.3% Fines	· 27 4%		Sieve	
-	4". 0 i				(10)	٠,		110.10.0	770, 014701. 0.0	770, G aria. 1	2.070, 1 11100	J. 27.170			
_	4" Casing	S-4	ss	6-8	11-7-6	3-10		S-4: Dark	Grey mf SAN	ID, little Silt,	trace fine (Gravel, wet (S	SM)		
_					(100	%)			•			, ,	,		
_													(==\)	_	
_		S-5	SS	8-10	(100)			S-5: Dark	Grey mf* SA	ND, trace Si	It, trace fine	Gravel, wet	(SP)		
10					(,									
_ :		S-6	ss	10-12	6-8-9	9-6		S-6: Brov	vn cmf SAND,	little Silt, tra	ace fine Gra	vel, wet (SM))		
_	4" Casing				(1009	%)									
_															
_															
_								 -							
- 45															
15		 S-7	ss	15-17	5-6-8	-11		S-7: Brov	vn SILT and fi	ne Sand we	et (ML)			Pass No	,
_				10 17	(79%				6%, Fines: 59.3		/t (IVIL)			200	
_															
-															
_								L						. 🚽	
_															
20				00.00	0.046			005.	0 5 04	ND 0		0 1 1	(014)		
_		S-8	SS	20-22	2-3-16			5-8: Dark	Grey fine SA	ND, some S	olit, trace fin	e Gravei, wet	(SIVI)		
_						,									
-															
_														_	
25															
-		S-9	SS	25-27	4-5-5 (96%			S-9: Grey	y-Brown CLAY 2%, LL: 28, PL	' & Silt, som	e fine Sand	, wet (CL)		Atterberg Limits	3
<u> </u>					(907	υ <i>)</i>		vvO. 21.2	. /v, LL. ZO, PL	. тә, гт. У				Liiillis	
-							1////	Bottom o	f Borehole @	27 ft.				7	

BORING NO.: **B-1**



Engineering Progress

BORING LOG

												BORIN	IG NO.:	B-2
												SHEET	г <u>1</u>	OF1
PROJEC	CT NO.:	20-	1052	PRO	JECT: N	NJDC	A Ge	otechnical E	ngineer for	Mitigation A	Assistance	Program DA	TE:	5/17/21
PROJEC	T LOCATI				Fairfield				ORING LOCA			ece Road, W		
DRILLIN	G EQUIPN	/ENT:		CME 5	55	ANG	GLE:	-90.0 DI	R.:	- ELE	V.:	DATUI	M:	NAVD88
DRILLIN	G CONTR	ACTO)R:	В	oring Br	othe	rs, Ind	DF	RILLER:	R. Do	llar	INSPECTO	DR:	A. Radiola
	CASIN	VC one	HAMN	/ED		ı		CAMPLED 6	and HAMMER			GROUNDW	ATED LEVE	
Туре			Weig		Drop	 	уре	I.D.	Weight	Drop	Date	Time	Depth	Casing Depth
Auto		•	140	_	30"	_	UTO		140 lbs	30"	54.0	1	2004	Casing 2 spin
FJ Ste	el 4"	'					SS	1 3/8"						
Depth	CASING		;	SAMPLE			흔							Laboratory
Feet (Elev.)	Blows/ Foot	No.	Туре	Depth Feet	Blows (REC. [RQD	%)	Graphic Symbol		De	escription	Of Mater	ial		Tests
		S-1	SS	0-2	3-8-11 (42%			S-1: Brown	n fine SAND,	some Silt, I	ittle cf Grav	el, moist (SM)	
- -		S-2	SS	2-4	4-3-4	1-4		S-2: Browr	n-Tan fine SA	ND, some S	Silt, trace cf	Gravel, mois	st (SM)	
– - –			00	4.0	(33%				Sa - OAND -	- 1 O'll to	.	1 4415	-1-4 (014)	Dogo No
5	4" Casing	S-3	SS	4-6	9-3-4 (29%			WC: 22.19	fine SAND at 6, Fines: 36%	id Siit, trace	e fine Grave	l, mottling, m	oist (SIVI)	Pass No 200
- - -	4 Casing	S-4	ss	6-8	8-9-9 (63%			S-4: Grey-	Brown cmf S	AND, trace	Silt, trace fi	ne Gravel, w	et (SP)	
- - -		S-5	ss	8-10	11-6-10 (100°			S-5: Brown	n cmf SAND,	little Silt, tra	ace fine Gra	vel, wet (SM))	
10 - - -	4" Casing	S-6	SS	10-12	12-8- (100°			S-6: Same	as Above, w	ret (SM)				
- - 15 - - - -		S-7	SS	15-17	7-6-8 (29%			S-7: Brown	n SILT and m	f SAND, tra	ce fine Gra	vel, wet (ML)		
20 20 		S-8	SS	20-22	2-2-2 (100°	2-1 %)		S-8: Brown WC: 24.5%	n CLAY & Sil 6, Sand: 8.9%	t, trace fine 6, Fines: 91	Sand, wet (.1%, <2 μm	 CL) :: 14%		Sieve; Hydrometer
- - 25 - - -		S-9	SS	25-27	5-7-6 (79%				as Above, w					
	1				1			Bottom of	Borehole @ 2	27 ft.				

BORING NO.: **B-2**



TEST PIT INCH 20-1052 TEST PIT LOGS.GPJ MATRIX EGS.GDT 7/9/21

TEST PIT LOG

					IESI	PII LOG					-	
									TEST PIT	NO.:		'-1
									SHEET	1_	OF _	1
PROJEC	T NO.:	20-	1052	PROJECT: NJ	DCA Geotechnical En	gineer for Mitiga	tion Assist	ance Progr	ambate: _	5	/14/202	1
PROJEC	T LOC	ATION:			Fairfield, NJ		ELEV.:		TIME STA	ARTED	11:0	0:00 AN
TEST PI	Γ LOCA	TION:	104 B	ig Piece Road (Northeast Corner - C	rawl Space)	DATUM:	NAVD88	TIME FIN	ISHED	12:0	0:00 PM
CONTRA					ing Brothers, Inc.							
EQUIPM	ENT:		Bobo	at E55	OPERATOR: _	Steve		INSPECT	OR:	A. I	Bangar	
Depth Inches (Elev)	No.	Depth Inches	Graphic Symbol		Des	scription Of Ma	aterial				- 1	oratory
15 		0-12 12-56		Brown mf SAN	te encountered at 48" b	Gravel (SM)	rom the fac	e of the wall	and extend	ds 8"		

TEST PIT NO.: TP-1



TEST PIT INCH 20-1052 TEST PIT LOGS.GPJ MATRIX EGS.GDT 7/9/21

TEST PIT LOG

					IESI	PII LOG						
									TEST PIT	NO.:		· <u>-2</u>
									SHEET	_1_	OF _	1
PROJEC	T NO.:	20-	1052	PROJECT:N	JDCA Geotechnical En	gineer for Mitiga	tion Assis	tance Progr	anbotATE: _	5	/14/202	:1
PROJEC	T LOC	ATION:			Fairfield, NJ		ELEV.:		TIME STA	ARTED	12:0	0:00 PM
					Road (East Wall - Craw							
CONTRA	CTOR:			Вс	oring Brothers, Inc.		GROUNE	WATER LE	VEL:			
EQUIPM	ENT:		Bobo	at E55	OPERATOR:	Steve		INSPECT	OR:	A. I	Bangar	
Depth Inches (Elev)	No.	Depth Inches	Graphic Symbol		De	scription Of Ma	aterial					oratory
5 - 10 - 15 - 20 - 35 - 40 - 45		0-12		Brown mf SA Top of concr	AND and Silt, some fine CAND and Silt, some fine CAND and Silt, some fine cete encountered at 37" lest pit @ 49 in. kfilled.	Gravel (SM)	from the fac	e of the wall	and extend	ds 12"		

TEST PIT NO.: **TP-2**

LOG NOTATION

Sample Classifications

SS = Split Spoon

NR = No Recovery

NX = Rock Core

SH = Shelby Tube

REC = Soil Recovery

RQD = Rock Quality Designation

Sand Classifications

c = Coarse

m = Medium

f = Fine

* = Predominant Grain Size

Soil Properties

WC = Water Content

PL = Plastic Limit

LL = Liquid Limit

PI = Plasticity Index

OC = Organic Content

LOG GRAPHICAL LEGEND

	Asphalt
P 6 4	Concrete
	Fill
××××	Topsoil
1	Well graded Gravel (GW)
000	Poorly graded Gravel (GP)
	Clayey Gravel (GC)
	Silty Gravel (GM)
	Well graded Gravel with Clay (GW-GC)
	Well graded Gravel with Silt (GW-GM)
	Poorly graded Gravel with Clay (GP-GC)
	Poorly graded Gravel with Silt (GP-GM)
2.7H K	Well graded Sand (SW)
	Poorly graded Sand (SP)
	Clayey Sand (SC)
	Silty Sand (SM)
	Well graded Sand with Clay (SW-SC)
	Well graded Sand with Silt (SW-SM)
	Poorly graded Sand with Clay (SP-SC)
	Poorly graded Sand with Silt (SP-SM)
	Lean Clay (CL)
	Silty Clay (CL-ML)
	Silt (ML)
	Organic Silt or Clay (Low Plasticity) (OL)
	Fat Clay (CH)
	Elastic Silt (MH)
*****	Organic Silt or Clay (High Plasticity) (OH)
	Peat (PT)
1.1.1.1	Decomposed Bedrock
	Bedrock
V/XV	

APPENDIX B SOIL CLASSIFICATION TABLES

MAJOR DIVISIONS		GROUP SYMBOLS	TYPICAL NAMES	FIELD IDENTIFICATION PROCEDURES (EXCLUDING PARTICLES LARGER THAN 3 IN. AND BASING FRACTIONS ON ESTIMATED WEIGHTS)		RGER THAN 3	INFORMATION REQUIRED FOR DESCRIBING SOILS		LABORATORY CLASSIFICATION CRITERIA					
1	2		3	4		5		6			7	200.000.000.000.000		
	ction is ize. eve size.)	Clean Gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixture, little or no fines.	Wide range in grain size and substantial amounts of all intermediate particle sizes.		of all intermediate particle sizes.		stantial amounts	For undisturbed soils add information on stratification, degree of compactness, cementation, moisture condition, and drainage characteristics.		s:	$C_u = \frac{D_{60}}{D_{10}}$ Greate $C_e = \frac{(D_{30})^2}{D_{10} \times D_{60}}$	er than 4 Between 1 and
SIZE.	rels coarse fraction is o. 4 sieve size. he No. 4 sieve siz	Clean Or	GP	Poorly graded gravels or gravel-sand mixture, little or no fines.		one size or a range ate sizes missing.	e of sizes with	moisture condition, and drainings characteristics.		follow	Not meeting all g requirements for			
it is larger than No. 200 sie Grav More than half of larger than No	Gravels More than half of coarse fraction is larger than No. 4 sieve size. used as equivalent to the No. 4 sieve size.)	th Fines amount of	GM	Silty gravels, gravel and silt mixtures.	Nonplastic fines (for identification	or fines with low on procedures see	plasticity ML below).	Give typical name; indicate approximate percentages of sand and gravel, maximum size; angularity, surface condition, and hardness of the coarse grains; local or geologic name and other pertinent descriptive information; and symbol in parentheses.		Atterberg limits belo um "A" line or was than 4		Above "A" li with P1 between 4 an		
	More that larg	Gravels with Fines (Appreciable amount of	GC	Clayey gravels, gravel and clay mixtures.	Plastic fines (for identification	on procedures see	CL below).			ain-size curve. I grained soils are iring use of dual	7	th han symbols.		
	Sands un half of coanse fraction is smaller than No. 4 sieve size. visual classification, the ¹ / ₄ -in, size may be	ean Sand e or no fines)	sw	Well-graded sands, gravelly sands, little or no fines.	Wide range in g of all intermedia	rain size and subs ate particle sizes.	stantial amounts		$C_s = \frac{D_{00}}{D_{10}} \text{ Gread}$ $C_c = \frac{(D_{30})^2}{D_{10} \times D_{00}}$ Not meeting all requirements for Atterberg limits below: Less than 4 Atterberg limits above was lime or save state of the mode of t		$Cu = \frac{D_{60}}{D_{10}}$ Greater $C_{e} = \frac{(D_{30})^{2}}{D_{10} \times D_{60}}$	than 6 Between 1 ar		
ible to t	iction is size.	Clean (Little or	SP	Poorly graded sands or gravelly sands, little or no fines.		one size or a range ate sizes missing.	e of sizes with	nuder fü		ovel and 300 sieve 37W, GF 37M, GC 30M, GC	Not meeting all g requirements for			
smallest vis	Sands of coarse fractio No. 4 sieve size classification. th	n Fines amount of	SM	Silty sands, sand-silt mixtures.	Nonplastic fines (for identification	or fines with low on procedures see	plasticity ML below).	Example: Silty sand, gravelly; about 20% hard, angular gravel particles \(^1/_2\)-in. maximum size; rounded and	ns as given u	entage of grant than No. 21	Atterberg limits above "A" line or P1 less than 4	Limits plotti in hatched zo with P1 between 4 ar		
No. 200 sieve size is about the smallest visible to the naked eye	More than half of No (For visual cla	Sands with Fines (Appreciable amount of fines)	SC	Clayey sands, sand-clay mixtures.	Plastic fines (for identification	on procedures see CL below).		subangular sand grains, coarse to fine; about 15% nonplastic fines with low dry strength; well compacted and moist in place; alluvial sand; (SM).	curve in identifying the fractions	Optermine perconfraction smalle Less than 5% More than 12 5% to 12%	Atterberg limits above "A" line with Pl greater than 7	7 are borderline cases requiri use of dual symbols.		
The No. 200 sieve s					Identification Procedure on Fraction Smaller than No. 40 Sieve Size.				n identif					
The No. 200					Dry Strength (Crushing Characteristics)	Dilatancy (Reaction to shaking)	Toughness (Consistency near PL)							
I	Silts and Clays quid limit is less	n 50	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.	None to slight	Quick to slow	None	For undisturbed soils add information on structure, stratification, consistency in undisturbed and	Use grain-size	Fo	LIQUID LIM PLASTICITY CI or laboratory classifi	IART		
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.	Medium to high	None to very slow	Medium	remolded states, moisture and drainage conditions	ם		fine-grained so			
	imit is		OL	Organic silts and organic silty clays of low plasticity.	Slight to medium	Slow	Slight	Give typical name; indicate degree and character of		89 60 Cm	nparing Soots at Equal Liquid L	Jesis		
	s Liquid limit is r than 50		МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.	Slight to medium	Slow to none	Slight to medium	plasticity; amount and maximum size of coarse grains; color in wet condition; odor, if any; local or geologic name and other pertinent descriptive		Plasticity Index	gluces and Dry Strength Increase Increasing Placeholity Index.	CHALi		
	and Clays	5	СН	Inorganic clays of high plasticity, fat clays.	High to very high	None	High	information; and symbol in parentheses.		20	CL (OH &		
8	Silts a		ОН	Organic clays of medium to high plasticity, organic silts.	Medium to high	None to very slow	Slight to medium	Example: Clayey silt, brown; slightly plastic; small percentage of fine sand; numerous vertical root holes; firm and dry in place; loess; (ML)		10 4 0 10	MIL 20 30 40 50 60	70 80 90		
Hig	ghly Organic So	oils	Pt	Peat and other highly organic soils.	Readily identific	ed by color, odor, prous texture	spongy feel and							

All sieve sizes on this chart are U.S. standard.
 Adopted by Corps of Engineers and Bureau of Reclamation, January 1952.

BURMISTER SOIL IDENTIFICATION METHOD

BURMISTER SOIL IDENTIFICATION METHOD

1. SOIL MATERIAL Composition, Gradation, and Plasticity Characteristics

a) Soil Components and Soil Fractions

Sieve	3"	1"	3/8"	No. 10)]	No. 30	No	. 60	No. 200	
				2 mm					0.076 mm	0.02 mm
Granular		GRAV	EL			SAND			SI	LT
Component Fractions	coarse	mediu	m f	ine co	arse	medi	ım	fine	coarse	fine
Clay Soil									CLAY	-SOIL
Components									Defined and	Named on a
									Plastici	ty Basis

Identifying Terms for Granular Soils Composition and Proportion Terms for Components

Component		Proportion	Defining Range		
		<u>Terms</u>	of Percentages		
Principal Compone (all Uppercase)	nts- GRAVEL, SAND, SILT		50% or more		
Minor Components	s- Gravel	and	35 to 50%		
	Sand	some	20 to 35%		
	Silt	little	10 to 20%		
		trace	1 to 10%		
Gradation Terms fo	or Granular Soils	ORGANIC SOILS			
coarse to fine	all fractions more than 10%	Plastic	city Basis, as		
coarse to medium	fine less than 10%				
medium to fine	coarse less than 10%	Organi	c SILT, H. PI		
medium	coarse and fine less than 10%				
fine	coarse and medium less than 10%	Organi	ic SILT, L. PI		
PLUS or MINUS sig	gns used to indicate upper or lower limits.				

Identifying Terms for CLAY SOILS. Plasticity Basis for Combined Silt and Clay Components, Expressing the Relative Dominance of Clay

Overall Plasticity	Plasticity Index	Principal Component	Minor Component
Non-Plastic	0	SILT	Silt
Slight	1 to 5	Clayey SILT	Clayey Silt
Low	5 to 10	SILT & CLAY	Silt & Clay
Medium	10 to 20	CLAY & SILT	Clay & Silt
High	20 to 40	Silty CLAY	
Very High	more than 40	CLAY	

Example: Soil 60% coarse to fine Sand, 25% medium to fine Gravel, 15% Clayey Silt and color-brown.

Identification: Br. coarse to fine SAND, some medium to fine Gravel, little Clayey Silt.

References: 1) D. M. Burmister, "Principles and Techniques of Soil Identification" 29th Highway Research Board Proceedings, 1949.

 "Identification and Classification of Soils – An appraisal and Statement of Principles", ASTM Special Technical Publication No. 113, 1951.

Field Classification of Soil Using the USCS

Apparent Density of Coarse-Grained Soils

SPT N-Value (corrected)	Apparent Density
0 - 4	Very loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
> 50	Very Dense

Consistency of Fine-Grained Soils

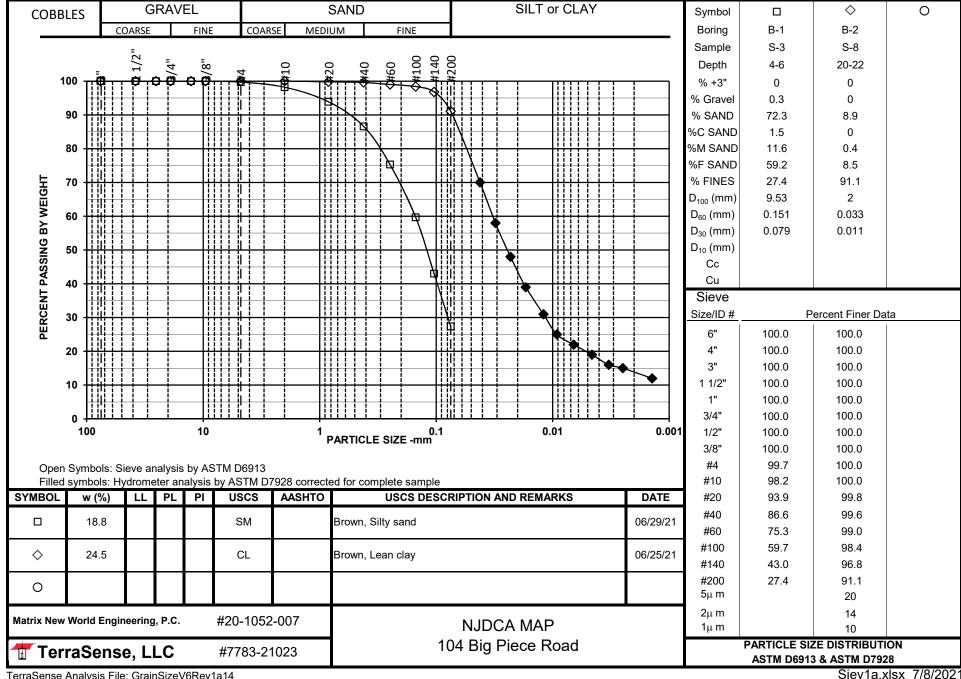
SPT N-Value (uncorrected)	Consistency	Compressive Strength (ksf)	Results of Manual Manipulation
< 2	Very Soft	< 0.5	Specimen (height = twice the diameter) sags under its own weight; extrudes between fingers when squeezed
3 - 4	Soft	> 0.5 - 1.0	Speciment can be pinched in to between the thumb and forefinger; remolded by light finger pressure
5 - 8	Medium stiff	> 1.0 - 2.0	Can be imprinted easily with fingers; remolded by strong finger pressure
9 - 15	Stiff	> 2.0 - 4.0	Can be imprinted with considerable pressure from fingers or indented by thumbnail
16 - 30	Very stiff	> 4.0 - 8.0	Can be barely imprinted by pressure from the fingers or indented by thumbnail
> 30	Hard	> 8.0	Cannot be imprinted by fingers or difficult to indent by thumbnail

APPENDIX C GEOTECHNICAL LABORATORY TESTING RESULTS

Matrix New World Engineering, P.C. #20-1052-007 NJDCA MAP - 104 Big Piece Road LABORATORY TESTING DATA SUMMARY

BORING	SAMPLE	DEPTH			IDEN	NTIFICAT	ION TESTS	S		REMARKS
			WATER	LIQUID	PLASTIC	PLAS.	USCS	SIEVE	HYDROMETER	
NO.	NO.		CONTENT	LIMIT	LIMIT	INDEX	SYMB.	MINUS	% MINUS	
							(1)	NO. 200	2 μm	
		(ft)	(%)	(-)	(-)	(-)		(%)	(%)	
B-1	S-3	4-6	18.8				SM	27.4		
B-1	S-7	15-17	26.5				ML	59.3		
B-1	S-9	25-27	27.2	28	19	9	CL			
B-2	S-3	4-6	22.1				SM	36		
B-2	S-8	20-22	24.5				CL	91.1	14	

Note: (1) USCS symbol based on visual observation and Sieve and Atterberg limits reported.



APPENDIX D FEMA NFIP ELEVATION CERTIFICATE

U.S. DEPARTMENT OF HOMELAND SECURITY Federal Emergency Management Agency National Flood Insurance Program

OMB No. 1660-0008 Expiration Date: November 30, 2022

ELEVATION CERTIFICATEImportant: Follow the instructions on pages 1–9.

Copy all pages of this Elevation Certificate and all attachments for (1) community official, (2) insurance agent/company, and (3) building owner.

	SECTION A – PROPERTY INFORMATION								
A1. Building Owner	er's Name					Policy Numb	per:		
A2. Building Stree Box No. 104 Big Piece Roa	,	cluding Apt., Unit, Suit	e, and/o	r Bldg. No.) o	r P.O. Route and	Company N	AIC Number:		
City	vld.			State	201	ZIP Code			
Town of Fairfie		nd Block Numbers, Ta	N Dorool	New Jer		07004-1210			
Block 5101, Lot 34		TIG BIOCK NUTIDETS, To	ax Faicei	i Number, Let	gai Description, et				
A4. Building Use (A4. Building Use (e.g., Residential, Non-Residential, Addition, Accessory, etc.) Residential								
A5. Latitude/Longi	A5. Latitude/Longitude: Lat. N40°52'55" Long. W74°19'08" Horizontal Datum: NAD 1927 X NAD 1983								
A6. Attach at leas	t 2 photograp	hs of the building if the	e Certific	ate is being u	sed to obtain floo	d insurance.			
A7. Building Diagr	am Number	4							
A8. For a building	with a crawls	pace or enclosure(s):							
a) Square foo	tage of crawl	space or enclosure(s)			793.00 sq ft				
b) Number of	permanent flo	ood openings in the cr	awlspace	e or enclosure	e(s) within 1.0 foo	t above adjacent gra	ide <u>0</u>		
c) Total net ar	c) Total net area of flood openings in A8.b 0.00 sq in								
d) Engineered flood openings?									
A9. For a building	A9. For a building with an attached garage:								
a) Square foo	tage of attach	ned garage		0.00 sq ft					
b) Number of	permanent flo	ood openings in the at	tached g	arage within	1.0 foot above adj	acent grade 0			
c) Total net ar	ea of flood of	penings in A9.b		0.00 sq	in				
d) Engineered	l flood openin	gs? ☐ Yes ☐ N	No						
	SE	CTION B – FLOOD	INSURA	NCE RATE	MAP (FIRM) INF	ORMATION			
B1. NFIP Commur Fairfield, Township		Community Number		B2. County	Name		B3. State		
Fairneid, Township	, 01			Essex			New Jersey		
B4. Map/Panel Number	B5. Suffix	B6. FIRM Index Date	Effe	RM Panel ective/ vised Date	B8. Flood Zone(s)	B9. Base Flood E (Zone AO, use	levation(s) e Base Flood Depth)		
34013C0014	G	04-03-2020	04-03-2		AE	174 (NAVD)			
B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in Item B9:									
B11. Indicate elevation datum used for BFE in Item B9: NGVD 1929 X NAVD 1988 Other/Source:									
B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? Yes No									
Designation Date: CBRS OPA									
	Designation Date UDRS UDRS								

ELEVATION CERTIFICATE

OMB No. 1660-0008 Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy the corresponding	information from Sec	tion A.	FOR INSURANCE COMPANY USE				
Building Street Address (including Apt., Unit, Suite, and/or 104 Big Piece Road	Bldg. No.) or P.O. Rout	e and Box No.	Policy Number:				
City State Town of Fairfield New		Code 4-1210	Company NAIC Number				
SECTION C – BUILDING ELE	EVATION INFORMAT	ION (SURVEY RE	:QUIRED)				
C1. Building elevations are based on: *A new Elevation Certificate will be required when concern to the complete Items C2.a—h below according to the build Benchmark Utilized: CORS Network NGS Monument Indicate elevation datum used for the elevations in item NGVD 1929 X NAVD 1988 Other/S Datum used for building elevations must be the same	nstruction of the buildir /E, V1–V30, V (with BF ing diagram specified in ts Vertical Datum: ems a) through h) below ource:	E), AR, AR/A, AR/A I Item A7. In Puerto NAVD 1988 V.	AE, AR/A1–A30, AR/AH, AR/AO. o Rico only, enter meters. Check the measurement used.				
a) Top of bottom floor (including basement, crawlspa	ace, or enclosure floor)		167.2 X feet meters				
b) Top of the next higher floor	0.4.7		173.2 🗷 feet 🗌 meters N/A 🦷 feet 🦳 meters				
c) Bottom of the lowest horizontal structural membed) Attached garage (top of slab)	r (V Zones only)		169.0 X feet meters				
e) Lowest elevation of machinery or equipment serv (Describe type of equipment and location in Com-			173.5 X feet meters				
f) Lowest adjacent (finished) grade next to building	(LAG)		168.3 X feet meters				
g) Highest adjacent (finished) grade next to building	(HAG)		169.3 X feet meters				
 h) Lowest adjacent grade at lowest elevation of decistructural support 	c or stairs, including		167.8 X feet meters				
SECTION D – SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION							
This certification is to be signed and sealed by a land sur I certify that the information on this Certificate represents statement may be punishable by fine or imprisonment un	my best efforts to interp der 18 U.S. Code, Sect	oret the data availa ion 1001.	ble. I understand that any false				
Were latitude and longitude in Section A provided by a lic		× Yes □ No	Check here if attachments.				
Certifier's Name Frank J. Barlowski	License Number 24GS03973500						
Title Professional Land Surveyor			Place				
Company Name Matrix New World Engineering, Land Surveying and Arch	itecture, P.C.		Seal				
Address 442 State Route 35, Second Floor			Here				
City Eatontown	State New Jersey	ZIP Code 07724					
Signature	Date	Telephone	Ext.				
Copy all pages of this Elevation Certificate and all attachments for (1) community official, (2) insurance agent/company, and (3) building owner.							
Comments (including type of equipment and location, per C2(e): Hot water heater on the first floor Elev = 173.5'(NA		cks on first floor					

ELEVATION CERTIFICATE

OMB No. 1660-0008 Expiration Date: November 30, 2022

IMP	ORTANT: In these spaces, copy the correspond	FOR INSURANCE COMPANY USE					
	ding Street Address (including Apt., Unit, Suite,	Policy Number:					
	04 Big Piece Road						
City	vn of Fairfield	State New Jersey	ZIP Code 07004-1210	Company NAIC Number			
				REQUIRED)			
	SECTION E – BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)						
For Zones AO and A (without BFE), complete Items E1–E5. If the Certificate is intended to support a LOMA or LOMR-F request, complete Sections A, B,and C. For Items E1–E4, use natural grade, if available. Check the measurement used. In Puerto Rico only, enter meters.							
E1.	E1. Provide elevation information for the following and check the appropriate boxes to show whether the elevation is above or below the highest adjacent grade (HAG) and the lowest adjacent grade (LAG).						
	Top of bottom floor (including basement, crawlspace, or enclosure) is		feet mete	rs			
	 Top of bottom floor (including basement, crawlspace, or enclosure) is 		feet mete	rs above or below the LAG.			
E2.	For Building Diagrams 6–9 with permanent floot the next higher floor (elevation C2.b in the diagrams) of the building is	od openings provided	in Section A Items 8 and/o				
E3.	Attached garage (top of slab) is		feet _ mete	rs above or below the HAG.			
E4.	Top of platform of machinery and/or equipmen servicing the building is	t	feet mete	rs above or below the HAG.			
E5.	Zone AO only: If no flood depth number is ava floodplain management ordinance? Yes			ccordance with the community's certify this information in Section G.			
	SECTION F - PROPERTY	OWNER (OR OWNER	'S REPRESENTATIVE) C	ERTIFICATION			
The property owner or owner's authorized representative who completes Sections A, B, and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. The statements in Sections A, B, and E are correct to the best of my knowledge.							
Property Owner or Owner's Authorized Representative's Name							
Add	dress	C	ity S	tate ZIP Code			
Sig	nature	D	ate Te	elephone			
Cor	nments						
				Check here if attachments.			

ELEVATION CERTIFICATE

OMB No. 1660-0008 Expiration Date: November 30, 2022

MPORTANT: In these spaces, copy the corre	FOR INSURANCE COMPANY USE					
Building Street Address (including Apt., Unit, St 104 Big Piece Road	Policy Number:					
City Town of Fairfield	State New Jersey	ZIP Code 07004-1210		Company NAIC Number		
SECTIO	N G – COMMUNI	TY INFORMATION (OPT	IONAL)			
The local official who is authorized by law or or Sections A, B, C (or E), and G of this Elevation used in Items G8–G10. In Puerto Rico only, en	Certificate. Compl	eter the community's flood ete the applicable item(s)	Iplain mar and sign	nagement ordinance can complete below. Check the measurement		
The information in Section C was taken from other documentation that has been signed and sealed by a licensed surveyor, engineer, or architect who is authorized by law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)						
A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.						
G3. The following information (Items G4–	G10) is provided for	or community floodplain r	nanageme	ent purposes.		
G4. Permit Number	G5. Date Permit	Issued		Date Certificate of compliance/Occupancy Issued		
G7. This permit has been issued for:	New Construction	n Substantial Improve	ement			
G8. Elevation of as-built lowest floor (including of the building:	g basement)		feet	meters Datum		
G9. BFE or (in Zone AO) depth of flooding at t	the building site: _		feet	meters Datum		
G10. Community's design flood elevation:	-		feet	meters Datum		
Local Official's Name		Title				
Community Name		Telephone				
Signature		Date				
Comments (including type of equipment and loc	cation, per C2(e), if	applicable)				
				Check here if attachments.		

BUILDING PHOTOGRAPHS

ELEVATION CERTIFICATE

See Instructions for Item A6.

OMB No. 1660-0008

Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy	FOR INSURANCE COMPANY USE		
Building Street Address (including Ap 104 Big Piece Road	Policy Number:		
City	State	ZIP Code	Company NAIC Number
Town of Fairfield	New Jersey	07004-1210	

If using the Elevation Certificate to obtain NFIP flood insurance, affix at least 2 building photographs below according to the instructions for Item A6. Identify all photographs with date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View." When applicable, photographs must show the foundation with representative examples of the flood openings or vents, as indicated in Section A8. If submitting more photographs than will fit on this page, use the Continuation Page.



Photo One

Photo One Caption Front View

Clear Photo One



Photo Two

Photo Two Caption Rear View

Clear Photo Two

BUILDING PHOTOGRAPHS

ELEVATION CERTIFICATE

Continuation Page

OMB No. 1660-0008

Expiration Date: November 30, 2022

IMPORTANT: In these spaces, copy to	FOR INSURANCE COMPANY USE		
Building Street Address (including Apt., 104 Big Piece Road	Policy Number:		
City	State	ZIP Code	Company NAIC Number
Town of Fairfield	New Jersey	07004-1210	

If submitting more photographs than will fit on the preceding page, affix the additional photographs below. Identify all photographs with: date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View." When applicable, photographs must show the foundation with representative examples of the flood openings or vents, as indicated in Section A8.



Photo Three

Photo Three Caption Right Side View

Clear Photo Three



Photo Four

Photo Four Caption Left Side View

Clear Photo Four